



# MANDATORY WOOD FRAME SOFT-STORY RETROFIT PLAN REVIEW LIST-DIVISION 93 (2017 LABC)

Plan Check Submittal Date: \_\_\_\_\_ Expiration Date: \_\_\_\_\_

PCIS Application Number: \_\_\_\_\_ Plan Check No.: \_\_\_\_\_

Address \_\_\_\_\_

Applicant Name: \_\_\_\_\_ Phone No.: \_\_\_\_\_

Plan Check Engineer: \_\_\_\_\_ Email: \_\_\_\_\_

Phone No: \_\_\_\_\_ firstname.lastname@lacity.org

Plan Check Supervisor: \_\_\_\_\_ Email: \_\_\_\_\_

Phone No: \_\_\_\_\_

**Your feedback is important to us.** Please visit <http://ladbs.org/our-organization/customer-services/customer-feedback-form> to complete the Customer Survey.

### INSTRUCTIONS FOR PROCEEDING WITH THE PLAN CHECK (PC) PROCESS:

1. Review all the items selected as marked  on this Plan Review List and the notes marked on the set of plans and calculations.
2. Address each item and revise the plans and/or structural analysis accordingly. **To prevent time delays, prepare a written response, addressing each item and referencing the location on the revised plans and/or calculation package.** For any questions related to this plan review, please e-mail or call the plan check engineer.
3. Once all corrections have been addressed, please email the plan check engineer with a copy of the written response. A verification appointment will then be scheduled. **Verification of corrections is done by appointment only.**
4. For the verification appointment, bring the originally checked set of plans and calculations, the revised sets of plans and calculations, the written response, and this Plan Review List. **Failure to supply all necessary documents will result in appointment postponement or cancellation and additional fees may apply.**
5. During the appointment, the plan check engineer will go over the corrections and comments. Once all the items have been corrected to comply with the code requirements and clearances are obtained, the permit will be ready to be issued.

### IMPORTANT ITEMS TO READ:

1. Your early attention is highly recommended for the approval process from other Departments as listed in the Clearance Summary Worksheet due to possible time delays.
2. The permit application will expire 18 months from the plan check submittal date.
3. Please be advised that the permit will be issued upon verification of compliance with the corrections included herein. The approval of plans does not permit the violation of any section of the Building Code, or other ordinance or state law.
4. Referenced numbers at the end of each correction are code sections of the 2017 edition of the Los Angeles Codes, the current Zoning Code and enacted code amendments thereafter.

**The following documents are available online, [www.ladbs.org](http://www.ladbs.org), to provide guidance and assistance in compliance to Division 93 of the Los Angeles Municipal Code (LAMC). Review the following checked documents and revise the plans and calculations accordingly.**

- |  |   |
|--|---|
| <input type="checkbox"/> Mandatory Wood Frame Soft-story Retrofit, Structural Design Guidelines <b>P/BC 2014-137</b><br><input type="checkbox"/> Mandatory Earthquake Hazard Reduction in Existing Wood-Frame Buildings with Soft, Weak or Open-Front Walls <b>Ordinance 183893 / 184081</b><br><input type="checkbox"/> Standard Quality Assurance Plan for Steel Moment Frames | <input type="checkbox"/> Structural Observation Report Form <b>Form.08</b><br><input type="checkbox"/> Foundation for Expansive Soils <b>P/BC 2014-116</b><br><input type="checkbox"/> Summary of Parking Regulation <b>P/ZC 2002-011</b><br><input type="checkbox"/> Parking Lot Design <b>P/ZC 2002-001</b> |
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## **PART I: GENERAL REQUIREMENTS**

### **A. PERMIT APPLICATION**

1.  Provide a fully dimensioned plot plan to scale, in ink, and copy onto the attached "Plot Plan Sheet".
2.  Project valuation is revised to \$ \_\_\_\_\_.  
Pay additional plan check fees of \$ \_\_\_\_\_.
3.  Provide complete and correct legal description (Tract, Block, Lot, and Grant Deed). Provide complete information for applicant, owner, engineer, architect, and contractor.
4.  The permit application must be signed by the property owner, a licensed contractor, or an authorized agent at the time the permit is to be issued:
5.  For owner-builder permits: Owner's signature can be verified with owner's driver license. Owner's representatives must present owner's approval with a notarized letter from the owner.
6.  For contractor building permits: Prior to the issuance of a building permit, the contractor shall have the following:
  - a.  Certificate of Worker's Compensation Insurance made out to the Contractors State License Board.
  - b.  Notarized Letter of Authorization for agents.
  - c.  Copy of Contractors State License or pocket ID.
  - d.  Copy of City of Los Angeles business tax registration certificate or newly paid receipt for one.

#### 7. CLEARANCES

Obtain all the clearances noted on the attached "**Clearance Summary Worksheet**". To prevent any time delay, please go to each listed agency immediately for sign-offs. Each agency's process of approval may take some time. Note that all conditions of approval must be met to obtain the permit and/or final inspection approval.

### **B. ADMINISTRATION**

1.  Each sheet of the architectural and structural plans must bear the signature, license number and expiration date of an architect or engineer licensed in the State of California.
2.  The address of the building is required on all plans. Name / address of the owner is required on first sheet. The name and address of the consultants are required on their plans.
3.  Two sets of plans will be required during permit issuance. Plans must be: **106.3.2.2, 106.3.3**
  - a.  Quality blue or black line drawings with uniform and light background color.
  - b.  Min. 11" x 17" size with minimum 1/8" lettering size.
  - c.  Sticky back details must produce prints without contrasting shades of background color.
4.  The final set of plans must be stamped and approved by (Fire Dept.), (\_\_\_\_\_).
5.  Provide a complete description for the entire scope of work on the plans.
6.  Provide the following with each set of plans:

<input type="checkbox"/> Floor Plans	<input type="checkbox"/> Four Elevations
<input type="checkbox"/> Construction Section	<input type="checkbox"/> Foundation Plans
<input type="checkbox"/> Framing Plans	<input type="checkbox"/> Structural Details
7.  Remove all plans, details or notes that do not pertain to the project.
8.  Provide a fully dimensioned plot plan to scale showing:

<input type="checkbox"/> Yard reduction due to retrofit	<input type="checkbox"/> Legal Description
<input type="checkbox"/> Building Lines	<input type="checkbox"/> Alley / Street Center
<input type="checkbox"/> Parking Space	<input type="checkbox"/> Area of Work
<input type="checkbox"/> Use/Size of all Buildings	<input type="checkbox"/> Property Lines

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## **PART II: ZONING CODE REQUIREMENTS**

(Allow time for discretionary approval process from City Planning if zoning requirements can't be met.)

1.  Show on plans that no required parking spaces have been compromised or removed. Show that the existing backup aisles and turning radius will be maintained and not affected. **12.21A5(b)**
2.  Plans show a reduction in parking. Provide ( ) paved parking spaces per latest Certificate of Occupancy or latest permit on record noting the required parking. **12.21A4, 12.21A5**
3.  Projection of ( ) into the ( ) yard/passageway is not permitted or limited to ( ). **12.22C20**
4.  Relocated and/or new parking spaces shall comply with current Zoning Code requirements. Comply with parking design standards. **12.21A.5 (h)**
5.  Plans shall be drawn to scale (around 1/8"=1'), showing aisle widths, circulation driveway, stall widths, and stall width increase for obstructions and end stall conditions. **12.21A5**
6.  Show that 7'6" stall width is maintained
7.  Clearly show on plan, existing fence wall surrounding the parking and driveway area. A 5'-9" high wall is required along the interior lot line(s) and 3' max high wall along the property line(s) fronting a street. A solid concrete or masonry wall of 6" thick construction is required for parking areas with over 4 cars **12.21A6(d),(e),(f)**
8.  Comply with **Zoning Information** file # \_\_\_\_\_.

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As a covered entity under Title II of the Americans with Disabilities Act, the City of Los Angeles does not discriminate on the basis of disability and, upon request will provide reasonable accommodation to ensure equal access to its programs, services and activities.

## **PART III BUILDING CODE REQUIREMENTS**

### **A. GENERAL PLANS**

1.  Provide table of contents in the calculation package, identifying the pages of each topic, i.e., seismic mass, base shear, shear transfer, drag strut, and foundation design.
2.  Identify and reference all sections and details as to their location on the plan and elevation views.
3.  Include the following summary in the calculation package for wall line along gridline(s) \_\_\_\_\_ and \_\_\_\_\_.
  - a.  Number of stories
  - b.  Tributary Area
  - c.  Tributary Width – ½ of the distance to next shear wall line plus length of cantilever (if any)
  - d.  Tributary Length
  - e.  Roof Dead Load
  - f.  Dead Load of each floor
  - g.  Partition load
  - h.  Exterior wall load
  - i.  Solid wall percentage
  - j.  Length – include perpendicular direction
  - k.  Height of the wall
4.  Justify that the wall line along gridline \_\_\_\_\_ or ( N , S , E , W ) elevation is/are not weak, soft, or open-front wall line. Provide pier/wall lengths on the elevation for field verification.
5.  Cross reference all calculations for joists, beams, shear walls, etc., to framing/floor plans.
6.  Submit structural calculations / design details for \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_
7.  Provide calculations and detailing for complete load path.
8.  A floor plan of the building is required to show the location of proposed retrofit. The floor plan must also show exiting scheme from the building in which moment frames will be placed.
9.  Structural plans, details, and calculations are required for the construction of moment frames.
10.  **The plans and/or structural analysis are incomplete. A resubmittal is required and additional fees will apply.**
11.  The engineer or architect shall provide the following statement on the approved plans:  
  
"I am responsible for designing this building's seismic strengthening in compliance with the minimum regulations of the Mandatory Earthquake Hazard Reduction. In Existing

Wood-Frame Buildings with Soft, Weak, or Open-Front Walls (LAMC Division 93)."

### **B. MATERIAL SPECIFICATION & INSPECTIONS**

1.  Specify the following items on plans:
  - a.  Type of soil and bearing value per
  - b.  Standard 2,500 psi concrete
  - c.  3000 psi min. for grade beams and piles/piers.
  - d.  Type and  $f_m$  of masonry units. Proportions of mortar and grout mixes.
  - e.  Type of Structural Steel, Structural Pipe, Tubing, Reinforcing bars.
  - f.  Grade, species, and moisture content of all lumber. Type and grade of plywood sheeting.
2.  Structural Observation is required for this project. The engineer of record shall prepare an inspection program, including the name(s) of the individuals or firms who will perform the work. The inspection program shall be shown on the first sheet of the structural plans. **1704**
3.  Include the following (Steel Moment Frame)
  - Connected components
  - Removal of backing bars
  - Placement of reinforcing fillets
  - Presence of continuity plates
  - Welding of continuity plates
  - Presence and type of doubler plates
  - Welding of doubling plates
  - Configuration and finish of access holes
  - Placement of beam stiffeners
  - Contour and finish of RBS profile
  - Placement of weld for web connection
  - Type and placement of bolts
  - Inaccessible conditions
4.  The following structural products shall comply with an approved Los Angeles City Research Report. Copy the conditions of approval onto the plans and show compliance with those conditions.
  - Hold-downs
  - Epoxy Anchors
  - Straps
  - Others such as \_\_\_\_\_
  - Prefab Shear Wall Panels
  - Expansion Anchor Bolts
  - Moment Frame
5.  **Add the following notes onto plans:**
  - a.  Contractors responsible for the construction of a seismic force resisting system/component listed in the "Statement of Special Inspection" shall submit a written statement of responsibility to the LADBS Inspectors and the owner prior to the commencement of work on such system or component. **1709.1 1**

- b.  **Pre-Construction Meeting:** Upon excavation and exposure of existing structural elements and connections and prior to installation of any new structural elements or members, the owner or owner's representative shall arrange a pre-construction meeting to be attended by the engineer or architect responsible for the structural design, contractor and the building inspector. The purpose of the meeting shall be to identify the major structural elements, connections and existing conditions that affect the vertical and lateral load systems of the structure and to review scheduling of the required observations.
- c.  Contractor is responsible for temporary shoring design by registered design engineer
- d.  Temporary shoring not to be removed until new foundation is capable of take gravity loads
- e.  Provide lead hole 40% - 70% of threaded shank diameter and full diameter for smooth shank portion.
- f.  Special inspection by a deputy inspector is required for shear panels where the fastener spacing of the sheathing is 4 inches on center or less.
- g.  A copy of the Los Angeles Research Report and/or conditions of listing shall be made available at the job site.
- h.  Provide standard details of splice, hooks and development length for all sizes of reinforcing bars.
- i.  Gas pipes not allowed in grade beam unless approval is obtained from gas company
- j.  Provide details for possible pipe intrusion
6.  Special Inspection or testing is required, The registered design professional in "responsible charge" shall include a "Statement of Special Inspections" noted on the plan as follows: **1705**
- a.  The owner or the registered design professional in responsible charge acting as the owner's agent shall employ one or more deputy inspectors to perform inspections during construction on the types of work that require special inspections
- b.  Continuous Special Inspection by a registered deputy inspector is required for:
- Field welding
  - Special moment resisting concrete frame
  - High load wood diaphragms
  - Driven deep foundation
  - Cast-in place deep foundations
  - Concrete strength  $f'_c > 2,500$  psi
  - High strength bolting
  - Sprayed-on fireproofing
  - High-lift grouting
  - \_\_\_\_\_ **1705**

- c.  Periodic Special Inspection by a registered deputy inspector is required for
- Wood shear wall -
  - Special moment resisting concrete frame
  - High load wood diaphragms
  - Shear panels
  - Cast-in place deep foundations
  - Concrete strength  $f'_c > 2,500$  psi
  - High strength bolting
  - Sprayed-on fireproofing
  - High-lift grouting
  - \_\_\_\_\_ **1705**
- d.  Foundation sills shall be naturally durable or preservative-treated wood. **2304.11.2.4**
- e.  Field welding to be done by welders certified by LADBS for
- Structural Steel
  - Reinforcing Steel
  - Light Gauge Steel
- f.  Shop welds must be performed in a LADBS licensed fabricator's shop.
- g.  LADBS licensed fabricator is required for
- Structural Steel
  - Glu-lam(grade/species) **2012 NDS Sup T'5-A**
  - \_\_\_\_\_

### C. LATERAL LOADS

1.  Earthquake design data - The following information related to seismic loads shall be shown on the plan. **1603.1.5, 1603.1.6**
- a.  Seismic importance factor I, and risk category.
- b.  Mapped spectral response accelerations,  $S_s$  and  $S_1$ .
- c.  Site class.
- d.  Spectral response coefficients,  $SD_s$  and  $SD_1$ .
- e.  Seismic design category.
- f.  Basic seismic-force-resisting system(s).
- g.  Design base shear. Total weight of building.
- h.  Seismic response coefficient(s),  $C_s$ .
- i.  Response modification factor(s), R.
- j.  Analysis procedure used.
- k.  Redundancy factor used.
- l.  The design load bearing value of soils
2.  Seismic Design Category (SDC) shall be based on LABC Table 1613.3.5(1) and Table 1613.3.5(2). When  $S_1$ , is greater than or equal to 0.75, the building shall be assigned to SDC E for Risk Category I, II, or III and assigned to SDC F for Risk Category IV.

3.  Design Base Shear

- a.  The design base shear shall be 75 percent of the value 12.8-1 of ASCE 7. **9309.2**
- b.  The upper system has a lower response modification coefficient, the design coefficients ( $R$ ,  $\Omega_0$ ,  $C_d$ ) of the upper system shall be used for the strengthening system. **ASCE 7 Sec.12.2.3.1**  
**Note:** However,  $R$  need not be less than 3.5 provided the strengthened structure will not have any vertical structural irregularities as defined in ASCE 7. The deflection amplification factor  $C_d$  value shall be 3 and overstrength factor,  $\Omega_0$ , shall be 3 which are consistent with the  $R$  value of 3.5 as specified in the seismic provisions of ASCE 7 for the lateral resisting system. Cantilever Column Systems shall still use the corresponding values.
- c.  Where a combination of different structural systems is utilized to resist lateral forces in the same direction, the value of  $R$  used for design in that direction shall not be greater than the least value of  $R$  for any of the systems utilized in that direction. **ASCE 7 Sec. 12.2.3.3.**

4.  Seismic Mass

- a.  Consider 10psf partition load per floor area. **ASCE 7 Sec.12.7.2**
- b.  Consider 15 psf roof and floor dead load
- c.  Include roof and floor dead load, psf, on drawings and next to framing plan on each level.
- d.  Consider ceiling framing and finishing weight for sloped roofs.
- e.  Add 15 psf to floor dead load for concrete topping.
- f.  Add note under the floor and roof dead load on plans that there is no concrete topping assumed in dead load calculations. Contractor to verify in field.
- g.  Consider 15 psf, exterior wall weight, per sf of wall, (direct or perpendicular to the grid line) or provide detailed exterior wall dead load calculations.
- h.  Consider 5 psf for existing or possible future installation of solar panels on the roof.
- i.  Consider exterior walls both in direction and perpendicular to the direction of soft wall line in seismic mass.
- j.  Consider 8 psf for existing or new stucco cover under the existing ceiling above parking area.
5.  Strengthening system with concrete walls or masonry walls or steel braced frames shall not be permitted **9309.3**
6.  Horizontal Structural Irregularities in buildings with 3 or more stories shall meet the additional requirements of ASCE 7 12.3.2.1, referenced in Table 12.3-1 "Horizontal Structural Irregularities" **9309.4**

7.  Vertical Distribution of Seismic Forces over the height of the structure shall be based on ASCE 7-10 Section 12.8.3.

8.  Anchorage Requirements for Buildings on Hillsides. Any portion of a building constructed on or into a slope steeper than one unit vertical in three units horizontal (33% slope), must comply with the provisions of LAMC Division 94. **9309.6**

9.  Story Drift Limitations

- a.  The calculated story drift for each retrofitted story shall not exceed the allowable deformation compatible with all vertical load-resisting elements and 0.025 time the story height **9309.7**
- b.  Calculate seismic drift based on deflections of each level with  $C_d$  and  $I$  factors using strength level forces in accordance with **ASCE 7 Sec.12.8.6.**
- c.  Verify that the stiffness of the strengthened system is greater than 70% of the Stiffness of the floor above or limit the drift ratio to 2.0%.

10.  Direction of Loading. Resisting systems used in multiple shear lines shall meet the requirements of **ASCE 7 Sec.12.5**

**D. HORIZONTAL DIAPHRAGM**

1.  Provide calculations and details to show diaphragm adequacy and shear transfer to the seismic resisting element.
- a.  Diaphragm cantilever is greater than 25% of diaphragm depth.
- b.  Diaphragm exceeds allowable values for existing diaphragms
2.  Diaphragm aspect ratio shall not exceed 3:1.
3.  Provide calculations and details on the plans for the sub-diaphragm and continuous cross-tie system required for all wood diaphragms providing lateral support to existing masonry or concrete walls. The spacing of continuous ties shall not exceed 40 ft. per LABC 1613.5.4.
4.  Ties, Continuity, and Collectors All parts of the structure shall be interconnected and capable of resisting the seismic force required. Provide calculations and details to show that collector elements, splices, and connections to resisting elements have the strength to resist the combined loads resulting from the load combinations with overstrength factor per ASCE 7-10 Sec.12.10.2.1 and Sec.12.14.7.3.
- a.  Where cantilever diaphragm is required to transfer seismic forces from above vertical resisting elements, the forces shall be added to  $F_{px}$
- b.  The design forces shall be increased to 25% 12.3.3.4
5.  Lumber and structural wood panel diaphragms shall not be considered as transmitting lateral forces by rotation. **SDPWS sec 4.2.5**
6.  For shear transfer from sole plate to rim use LTP5 instead of LTP4.

## **E. ALTERNATIVE DESIGN METHODS**

Pursuant to section 104.2.6, LADBS has approved the following alternative design methodologies whose objective is to improve the whole first story seismic performance. **9309.5**

1.  Appendix Chapter A4, 2012 International Existing Building Code with the following conditions:
  - a.  Comply with all standards as prescribed.
2.  ASCE 41-13, Seismic Evaluation and Retrofit of Existing Buildings with the following conditions
  - a.  Design to meet the Rehabilitation Objective (Section 1.4) (Life Safety Performance Level: S-3) for the BSE-1 earthquake hazard level.
  - b.  Retrofit strength need not exceed 1.3 times the strength of story above.
3.  FEMA P-807, Seismic Evaluation and Retrofit of Multi-Unit Wood-Frame Buildings with Weak First Stories with the following conditions.
  - a.  The entire story must be analyzed and designed.
  - b.  The spectral demand shall be  $0.5S_{MS}$ , calculated in accordance with ASCE 7-10 section 11.4 except that for sites in site class E, the value of  $F_a$  shall be taken as 1.3.
  - c.  Acceptable performance level shall be based on drifts corresponding to Onset of Strength Loss in the seismic force-resisting wood-frame elements.
  - d.  The maximum limit probability of exceedance (POE) for evaluation/retrofit design shall be 20%.
  - e.  Limit diaphragm ratio to 2:1.

## **F. WOOD SHEAR WALLS**

1.  Provide a shear wall schedule on the plans and specify the maximum design shear load for each shear wall type. Limit the design shear wall loads to those allowed by Code. Clearly indicate on the plans all plywood and drywall shear walls.
2.  The \_\_\_\_\_ is inadequate to resist lateral/ uplift forces. Show roof/floor diaphragm nailing, wall bracing, shear connections, tie downs and hold-down anchors. Submit lateral design.
3.  Provide shear connection details, properly referenced, at the top and bottom of all shear walls.
4.  The horizontal distribution of seismic shear to wood structural panel shear walls shall be in accordance to ASCE 7-10 Section 12.3.1.1.
5.  Provide LARR number for screws used in steel sheets or cold-formed steel framed walls sheathed with wood structural panels. Shear values for steel sheets or cold-formed steel framed walls sheathed with wood structural panels shall conform to the requirements in AISI-Lateral C2.2.1 and C2.2, respectively. Allowable values shall be in conformance with

T'C2.1-3, taking in to account the corresponding reductions per C2.1 (AISI-Lateral).

6.  Wood structural panel shear walls shall meet the story drift limitation of ASCE 7-10 Section 12.12.1 Conformance shall be determined by testing or calculations. Calculated deflection shall be determined according to equation 4.3-1 of SDPWS. **2305.3.2**
7.  Limit the height-width ratio of the plywood (wood structural panels) shear walls, perforated shear wall segments, perforated shear walls and shear wall piers to 2:1. Provide complete calculations (including deflection) and details for shear wall with openings. **T' 4.3.4 SDPWS-2008**
8.  The following applies to all shear walls with a shear values using allowable stress design (ASD) exceed 350 plf or load and resistance factor design (LRFD) exceed 500 plf. These walls shall be clearly identified on the plans and provided with the following: **2306.3**
  - a.  3 x studs and blocks for all framing members receiving edge nailing from abutting panels.
  - b.  1/2" edge distance from the panel edges and 3/8" from the edge of the connecting members.
  - c.  All wood structural panel joint and sill plate nailing shall be staggered at all panel edges.
9.  Provide calculations and details for drag struts and drag strut connections to shear walls.
10.  Provide referenced calculations showing the overturning moments in the shear wall segments.
11.  Per structural calculations, show size, location and embedment length of all anchor bolts (including HD bolt anchors) on foundation plan.
12.  Provisions under Section 1908 and Table 1908.2 do not apply to anchors installed in hardened concrete subject to earthquake loads. Justify the capacity of tie down bolt in concrete footing/wall/deck per ACI 318-11 Appendix D with factored design loads. **1908.1**
13.  When bolting to an existing footing, provide a copy of the LA Research Report approval for the type of bolt, allowable design loads and required edge distances. Deputy inspection is generally required.
14.  Provide LARR number for hold-down connectors. The capacity of hold-down connectors that do not consider cyclic loading of the product shall be reduced to 75% of the allowable earthquake load values. **2305.5**
15.  Hold-down straps - Include the following in design and detailing between floors:
  - a.  Design and detail straps installation when used as hold-downs across floor joists. Account for reduced number of nails across joist.
  - b.  Determine the allowable load based on the number of nails through the strap into the posts above and below

the "Dead Space" between floors. Specify the number of nails required.

- c.  Design and detail straps so that the minimum nail spacing will be provided when the strap nailing is combined with the shear wall edge nailing (i.e. provide 4x member wherever a strap and shear wall edge nailing occur or detail strap nailed over and through un-nailed plywood--show nailing pattern on plan)

16.  Add the following notes on the plans:

- a.  Hold-down connector bolts into wood framing require approved plate washers; and hold-downs shall be finger tight and ½ wrench turn just prior to covering the wall framing. Connector bolts into wood framing require steel plate washers on the post on the opposite side of the anchorage device. Plate size shall be a minimum of 0.299 inch by 3"x3". **2305.5**
- b.  Roof diaphragm nailing must be inspected before covering. Face grain of plywood shall be perpendicular to supports. Floor shall have tongue and groove or blocked panel edges. Plywood spans shall conform to Table 2304.7.
- c.  All diaphragm and shear wall nailing shall utilize common nails or galvanized box.
- d.  All bolt holes shall be drilled 1/32" to 1/16" oversized. **Sec11.1.2.2, 2012 NDS**
- e.  Hold-down hardware must be secured in place prior to foundation inspection.

## G. STEEL MOMENT FRAME

1.  Structural design drawings and specifications shall indicate the work to be performed, and include items required by AISC 341, AISC Code of Standard Practice for Steel Buildings and Bridges, and the 2017 LABC. Obtain a copy "Standard Quality Assurance Plan for Steel Moment Frames" sheet 1, 2, and 3; and include with the final set of plans.

2.  Clearly identify in the structural calculations and structural plans what type of steel moment frame system the building is designed for. Note on plan "The Lateral Force Resisting System for this building is a (Special Moment Frame) (Intermediate Moment Frame) (Ordinary Moment Frame).

3.  Steel Moment **ASCE 7 Table 12.2-1**

- a.  Ordinary Moment Frame is not permitted in Seismic Design Category D, E, or F unless the conditions below are met. (Refer to Structural Design Guidelines for Steel Moment Frames for more information.)
- i.  Structural Height of 35 feet
- ii.  Maximum 35 psf dead load per floor (including partitions)
- iii.  Exterior wall weight does not exceed 20 psf

b.  Intermediate Moment Frame is not permitted in Seismic Design Category E or F unless the conditions below are met (Refer to Structural Design Guidelines for Steel Moment Frames for more information.)

i.  Structural Height of 35 feet

ii.  Maximum 35 psf dead load per floor (including partitions)

iii.  Exterior wall weight does not exceed 20 psf of wall

4.  Variations/alterations to prequalified connections and connections qualified by cyclic tests, such as additional haunches or cover plates and additional welds, or deviations/alterations from the tested weld access hole configuration at moment connections are not permitted.

5.  Column Weak Axis (Skewed) (Dual Axis) moment connection is not permitted.

6.  For Reduced Beam Section (RBS) moment connections, comply with AISC 358 Section 5.3 for prequalification limits.

7.  For Bolted Unstiffened/Stiffened Extended End Plate (BUEEP, BSEEP) moment connections, comply with AISC 358 Section 6.3 for Prequalification limits. **Note:** SMF systems in direct contact with concrete structural slabs are not prequalified, unless they comply with

**AISC 358 6.2.**

8.  For other prequalified moment connections, comply with AISC 358 Section 7.3, 8.3, 9.3, and 10.3 for pre-qualification limits.

9.  Clearly identify on the plan the location and length of the expected plastic hinging zone. No welded, screwed, bolted, or shot-in attachment is permitted within this zone.

**AISC 341 Sec. I2-1 and D1-3**

10.  Column and beam members used in SMF or IMF shall meet the width-to-thickness ( $\lambda_{hd}$ ) limitations of T'D1.1 per AISC 341 Chapter D. **AISC 341 Sec.D1-1b**

11.  Provide a beveled transition detail where changes in thickness and width of flanges and webs occur in complete joint penetration groove welded column splices. **AWS D1.1 2.7.1, 2.16.1.1**

12.  Column splices shall be located 4 ft or more away from the beam-to-column flange connections, except:

**AISC 341 D2-5a**

a.  When the column clear height between beam-to-column flange connections is less than 8 ft. (2.4 m), splices shall be at half the clear height.

b.  Column splices with webs and flanges joined by complete-joint-penetration groove welds are permitted to be located closer to the beam-to-column flange connections, but not less than the depth of the column.

c.  Splices in composite columns.

13.  Splice plates or channels used for making web splices in the SFRS columns shall be placed on both sides of the column web. Detail this on the plan. **AISC 341 Sec.D2-5d**
14.  Groove welds for column splices shall be complete -joint-penetration groove welds that meet the requirement of AISC 341 A3-4b and I2-3 for demand critical welds. Weld tabs shall be re-moved upon completion of weld **AISC 341 E3-6a**
15.  Panel zone doubler plates shall comply with the requirements per AISC 341 E3-6e(3) as:
- Doubler plates in contact with the column web.
  - Spaced doubler plates.
  - Doubler plates used with continuity plates.
  - Doubler plates used without continuity plates.
16.  Continuity plate for SMF or IMF connections shall be detailed on the plan to match the prequalified connections in AISC 358 or connection prequalified in accordance with Section K1 or tested in accordance with Section K2
- AISC 341 Sec.E3-6f**
- For two-sided connections, the minimum thickness of continuity plate shall be equal to that of the thicker of beam flanges (or beam-flange connection plate). For one-sided connections, continuity plate thickness shall be at least one half of the thickness of the beam flange (or beam-flange connection plate).
  - Continuity plates shall be welded to column webs using groove welds or fillet welds. **AISC 341 Sec.E3-6f**
17.  Continuity plate for OMF connection shall be detailed on the plan in accordance with sections J10.1, J10.2 and J10.3 of AISC 360. **AISC 341 Sec. E1-6b**
18.  **SMF** When the beam-to-column moment ratio calculated using Equation (E3-1) is more than 2 (column remains elastic), the column flanges shall be laterally supported at the level of the top flanges of the beams.
- AISC 341 Sec.E3-4c**
19.  **SMF** When the beam-to-column moment ratio calculated using Equation (E3-1) is less than or equal to 2 (column does not remain elastic), the following requirements shall apply;
- Column flanges shall be laterally braced at the levels of both the top and bottom beam flanges. Stability bracing shall be either direct by attaching the lateral bracing element to the column flange at or near the desired bracing point to resist lateral buckling or, alternatively shall be indirect by attached to the column flanges, or rather act through the column web or stiffener plates. **AISC 341 Sec.E3-4c(1)**
  - Each column-flange lateral brace shall be designed for a required strength that is equal to 2 percent of the available beam flange strength  $F_y b t_{bf}$  (LRFD) or  $F_y b t_{bf} / 1.5$  (ASD), as appropriate. **AISC 341 Sec.E3-4c(1)**
20.  **SMF** Where unbraced connections occur in special cases such as two-story frames, atriums and similar architectural spaces. Comply with AISC 341 E3-4c(2) for unbraced Beam-to-Column connections to avoid lateral-torsional buckling of column. **AISC 341 Sec.E3-4c(2)**
21.  **SMF** Beams shall be braced to satisfy the requirements for highly ductile members per AISC 341 D1- 2b **AISC 341 Sec.E3-4b**
- Both flanges of beams shall be laterally braced or the beam cross section shall be torsionally braced.
  - The unbraced length between lateral supports shall not exceed  $0.086 r_y E / F_y$ . **AISC 341 Sec.D1-2b**
  - Lateral supports shall be provided near concentrated forces, changes in cross-section and other locations where analysis indicates that a plastic hinge will form during inelastic deformations.
  - The required strength of lateral bracing shall be  $M_r = R_y F_y Z$  (LRFD) or  $M_r = R_y F_y Z / 1.5$  (ASD), and the required strength of lateral bracing of each flange provided adjacent to plastic hinges shall be at least;  $P_u = 0.06 R_y F_y Z / h_o$  (LRFD) or  $P_u = (0.06/1.5) R_y F_y Z / h_o$  (ASD) and required stiffness shall meet the requirements of Appendix 6 of the AISC 360. **AISC 341 D1-2c**
  - The required strength of lateral bracing provided adjacent to plastic hinges for concrete encased composite beams shall be  $P_u = 0.06 M_{p,exp} / h_o$ .
22.  SMF the individual thicknesses of column webs and doubler plates, shall not be less than that specified in equation (E3-7) **AISC 341 E3-6e (2)**.
23.  Column members shall satisfy the requirements of AISC 341 D1-1 for highly ductile members. The compressive axial strength and tensile strength as determined using the load combinations stipulated in the 2017 LABC including the amplified seismic load **AISC 341 Sec.E3-5 and D1-4a**
24.  The measured flexural resistance of the connection, determined at the column face, shall equal at least  $0.80 M_p$  of the connected beam at an inter-story drift angle of 0.04 (SMF) or 0.02 (IMF) radians **AISC 341 Sec.E3-6b**
25.  The required shear strength,  $V_u$ , of the connection shall be based on load combinations per the 2017 LABC that include the amplified seismic load, where the amplified seismic load due to the effect of horizontal forces is  $E_{mh} = 2(1.1 R_y M_p) / L_h$ . **AISC 341 Sec.E3-6d**
26.  Provide calculations to show that the required shear strength,  $R_u$ , of the panel-zone is less than the design shear strength,  $\phi_y R_y$ , of the panel zone. **AISC 341 Sec.E3-6e**
27.  Members shall be sized to provide strong column/ weak beam in accordance with equation (E3-1) per AISC 341 E3-4a.

28.  Where column splice occurs, provide calculation to show that the required flexural and shear strength of column splices satisfy AISC 341 E3 - 6g, and AISC 341 D2 -5.
29.  The shape of web access holes shall be in accordance with subclause 6.10.1.2 of AWS D1.8/ D1.8M. Weld access hole quality requirements shall be in accordance with subclause 6.10.2 of AWS D1.8/D1.8M. **AISC 341 Sec.E1-6b(c)**
30.  Column and beam members are limited to wide flanges only, except for steel moment frame with "Symmetrical Shapes" in IB P/BC 2014-098 T'1,2,3.
31.  Fully restrained moment connections that are part of the SFRS shall satisfy at least one of the following requirements:

**AISC 341 Sec.E-6b**

- a.  The required flexural strength shall be equal to  $1.1R_yM_p$  (LRFD) or  $(1.1/1.5) R_yM_p$  (ASD). The required shear strength,  $V_u$  or  $V_a$ , shall be based on the load combinations stipulated in 2017 LABC including the amplified seismic load, where the amplified seismic load due to the effect of horizontal forces, including overstrength is  $E_{mh} = 2 (1.1R_yM_p)/L_{cf}$ .
- b.  Fully restrained moment connections shall be designed for a required flexural strength and a required shear strength equal to the maximum moment and corresponding shear that can be transferred to the connection by the system, including the effects of material overstrength and strain hardening.
- c.  Fully restrained moment connections between wide flange beams and the flanges of wide flange columns shall either satisfy the requirements of section E2.6 or E3.6, or satisfy all conditions listed on E1.6b(c)

32.  Provide width-to-thickness ratios of members for OMF to comply with AISC 360 requirements

**AISC 341 Sec.E1 - 5a**

33.  Provide detail for top of column bracing

**AISC 360 App. 6**

**H. STEEL CANTILEVERED COLUMN**

1.  Pole Structures shall include the effects of rotation and soil stiffness. Deflection calculations shall be based on approved Soils/Geology Report. Provide a copy of the approved report and the Department's approval letter.

**ASCE 7 Sec.12.13.6.1**

2.  P-Delta Effect Stress analysis of cantilevered columns shall have an effective length factor of 2.1 for the direction normal to the axis of the beam. **9309.9**

3.  Special cantilevered columns used as part of the seismic-force resisting system, shall comply with the following:

- a.  Maximum axial stress of 15% of allowable.

**AISC 341-10 Sec. E5, E6**

- b.  Columns designed as SCCS shall use R,  $C_d$  and  $\Omega_o$  factors as per Section G of T' 12.2-1 of ASCE 7-10.

- c.  The lowest R value shall be used in the same direction unless the building is a Risk Category I or II building that does not exceed 2 stories in height and light frame construction or flexible diaphragms are used.

**ASCE 7 Sec. 12.2.3.3**

- d.  Columns designed as SCCS shall comply with the requirements of T' D1.1 of AISC 341-10 for highly ductile members.

- e.  Columns designed as SCCS shall be braced to satisfy the moderately ductile bracing requirements of D1.2a of AISC 341-10.

- i.  Spacing:  $L_b = 0.17r_yE/F_y$

- ii.  Flexural Strength:  $M_r = R_yF_yZ$  (LRFD) or  $M_r = R_yF_yZ/1.5$  (ASD)

- f.  Label and Dimension Protected Zones

**I. SOFT/WEAK/OPEN WALL LINE**

1.  Wall lines with ratio of total full height length of piers of the first floor over the second floor less than 80% are weak and will require retrofit
2.  Only piers with  $h/b < 2$  will qualify for weak story calculation
3.  Provide dimensions of piers at the first and second floor to match calculations

**J. PERPENDICULAR TO OPEN WALL LINE**

4.  Justify wall line perpendicular to open wall line is not a weak and / or soft wall line or provide retrofit design.
5.  New lateral load bearing system to be designed to take seismic load of entire length of building not just the open area.
6.  Maximum allowable shear of Stucco walls with  $h/b < 2.0$  is 100pf.
7.  Limit the deflection of new lateral system to 1.25% for deformation compatibility with (E) stucco wall.

**K. FOUNDATION AND GRADING**

1.  All foundations shall be designed for expansive soil conditions unless a soil report is provided and approved by LADBS Grading Division (see Information Bulletin for more information). Provide details on foundation plans to comply with the requirements.

2.  Detail (and reference location on foundation plan) typical foundation sections for:

- a.  Bearing/shear walls
- b.  Spread and/or post pads
- c.  Grade beams
- d.  New to existing footing connections
- e.  Existing footing to new grade beam

3.  Concrete grade beams that are part of a Moment Frame and Cantilever column systems shall provide transverse reinforcement over a length equal to twice the member depth measured from the face of the supporting member. The spacing of such reinforcement shall not exceed: (a)  $d/4$ , (b) 6 times the diameter of the smallest primary flexural rebar excluding longitudinal skin rebar, or (c) 6 inches. The remainder of the grade beam shall have transverse reinforcement spaced not more than  $d/2$ .
- a.  ACI 318 Sec 21.3.4.2 and 21.3.4.3 for Intermediate Moment Frames
- b.  ACI 318 Sec 21.5.2, 21.5.3, and 21.5.4 for Special Moment Frames. Grade beams supporting Special Moment frames shall use A706 Reinforcing Steel
4.  Foundations designed to support cantilevered columns used as part of the SFRS shall have the strength to resist the load combinations with overstrength factor of Section 12.4.3.2 of ASCE 7.
5.  Provide justification that the proposed cantilever column/grade beam connection detail is fixed as assumed in your design.
6.  Run analysis including grade beam and calculate the deflection of moment frame/ cantilevered columns based on cracked moment of inertia of grade beam.
7.  Justify grade beam concrete bearing pressure at fixed base column flange location.
8.  Foundations shall be designed accordingly
- a.  Bearing capacity shall not exceed the allowable **1806.2**
- b.  Resist seismic overturning **1808.3.1**
- c.  Resist sliding **1605.1.1, 1806.3**
- i.  Sliding resistance is limited to half the dead load **1806.3.2**
- d.  Punching shear **ACI 318-08 Sec. 11.11.2.1**
9.  Individual spread footings shall be interconnected by concrete tie beams unless site class is confirmed not to be E or F by a LACITY approved soils report.
10.  If fixed end condition is assumed, the concrete grade beam shall be designed as a moment reaction.
11.  Anchorage to Concrete in shear and tensions should be designed for the following:
- a.  Ductility check must first be performed to provide a reasonable expectation, based on nominal strengths, that the anchor element will have yielded when ultimate load is reached  
**ACI 318-14 Sec. 17.2.3.4.3(a)**
- b.  The anchorage design to be controlled by ductile yielding of the attachment.  
**ACI 318-14 Sec. 17.2.3.4.3(b)/17.2.3.5.3**
- c.  The anchorage design to be controlled by the strength of a non-yielding attachment  
**ACI 318-14 Sec. 17.2.3.4.3(c)/ 17.2.3.5.3**
- d.  The anchor design strengths to be greater than or equal to the factored tension load inclusive of an  $\Omega_0$  overstrength factor in the earthquake component (E) of the factored load  
**ACI 318-14 Sec.17.2.3.4.3(d)/ 17.2.3.5.3.**
12.  Foundations with stem walls shall be reinforced with a minimum of two No. 4 bars at the top of the wall and two No. 4 bars at the bottom of the footing. **1908.1.8**
13.  Site drainage: Show on plans how concentrated drainage is being conveyed to the street via non-erosive devices. **7013.10**
14.  Add note on the plans:
- a.  If adverse soil conditions are encountered, a soils investigation report may be required. **1803.5.2**
- b.  No impact tools shall be permitted when removing existing footing. Saw cutting the existing footing only is allowed
- L. FIRE-LIFE SAFETY**
1.  **Fire resistance rated construction.** Steel beams and columns shall be protected as required for 1-hour protection. Where ceiling forms the protective membrane for fire-resistive assemblies (occupancy separations and rated roof/ceiling or floor/ceiling assemblies), the construction (floor joists) and their supporting horizontal structural members (beams) need not be individually fire protected except where such members support directly applied loads from more than one floor or roof. The required fire resistance shall not be less than that required for individual protection of members. **704.3**
2.  The means of egress has been compromised due to the strengthening system(s). Provide an alternative path of egress or relocate the strengthening system. **LABC Ch.10**
3.  Provide/dimension the required minimum 7'-0" clear height within the parking area. **406.4.1**



